



Plastic failure mechanisms of eccentrically loaded thin-walled cold-formed steel members

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Bauhaus Summer School, Weimar, 21 August - 2 September 2016



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What is a plastic failure mechanism?





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- Requires numerous iterations
- Complicated for nonstandard cross-sections



- •Thin-walled cold-formed (TWCF) steel structures are usually made of **class 4** sections which are prematurely susceptible to local or distortional buckling in the elastic range
- •Failure of these members in compression and bending is always initiated by local-global <u>interactive buckling</u> of plastic-elastic type
- •This interaction manifests as a **plastic failure mechanism**
- •Examination of the complete collapse response is needed, particularly for design under extreme loads

BREAKDOWN

- •Subject: thin-walled cold-formed (TWCF) steel members - class 4 sections
- •Loading: compression, bending and their interaction
- •**Phenomenon**: plastic-elastic interaction of local/distortional and global buckling $\rightarrow \underline{local \ plastic \ failure \ mechanism}$

•Analysis:

- FE simulation (Abaqus)
- Analytical (yield line analysis)







Buckling interaction

Two forms of interaction occur:

1. Interaction of **local** buckling modes

2. Local-global interactive buckling



Interaction of local buckling modes





- Local-global interaction
- •Factors that influence PFM
 - **1.** f_y
 - 2. Type of cross-section
 - 3. b/t
 - 4. Geometrical imperfections
 - 5. Residual stresses
- Not a linear superposition of M+N!!
- Hard to determine *a priori*





2. State of art review







- Murray and Khoo (1981) observed 5
 mechanisms that repeatedly manifested for plane
 channels in compression 5 basic mechanisms
- Various researchers performing experiments since the 1960s











- •Subject: lipped-channel (C-section)
- •Loading: eccentric compression







- Boundary conditions: pinned-pinned
- •Imperfections: eigenmode





<u>Input</u>

- • $fy = 355 \frac{N}{mm^2}$; $E = 210\ 000 \frac{N}{mm^2}$; $\nu = 0.3$ • $h = 150\ mm$ • $b = 50\ mm$ • $c = 15\ mm$
- •t = 1.5 mm
- •L = 450 mm





Negative eccentricities

Positive eccentricities



















Yield line analysis

theoretically zero-width lines of yielded material



(a) classical yield-line analysis of a simply supported plate (slab) with out-of plane load, yield-lines and patterns develop from *first-order* forces and moments



(b) generalized yield-line analysis of a simply supported plate with in-plane load, yield-lines and patterns develop from consideration of *second-order* forces and moments







Slab VS Beam/column differences

- 1. Out-of-plane vs in-plane loading
- 2. No way to predict the mechanism not necessarily the one with the lowest loading capacity!



Proposed mechanism model

- Model for failure in positive eccentricity
- Based on observations of FE simulation





Proposed mechanism model

- •Flanges and lips of the element are in compression
- •Web in tension
- •Yield lines in lips of the section were not accounted for
- •All walls are considered inextensible and incompressible - all deformation happens along the yield lines only



















5. Final remarks







- •Short TWCF columns in eccentric compression fail by forming a plastic collapse mechanism confirmed
- •Agreement of the theoretical and numerical curve is not very good the model needs more work
- •Detailed derivation of the mechanism model and load capacity expressions will be presented in a MSc thesis of the same title



Thank You for your attention!



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