

# ***Project 6***

## ***Wind-induced Vibrations of Long-span Bridges***

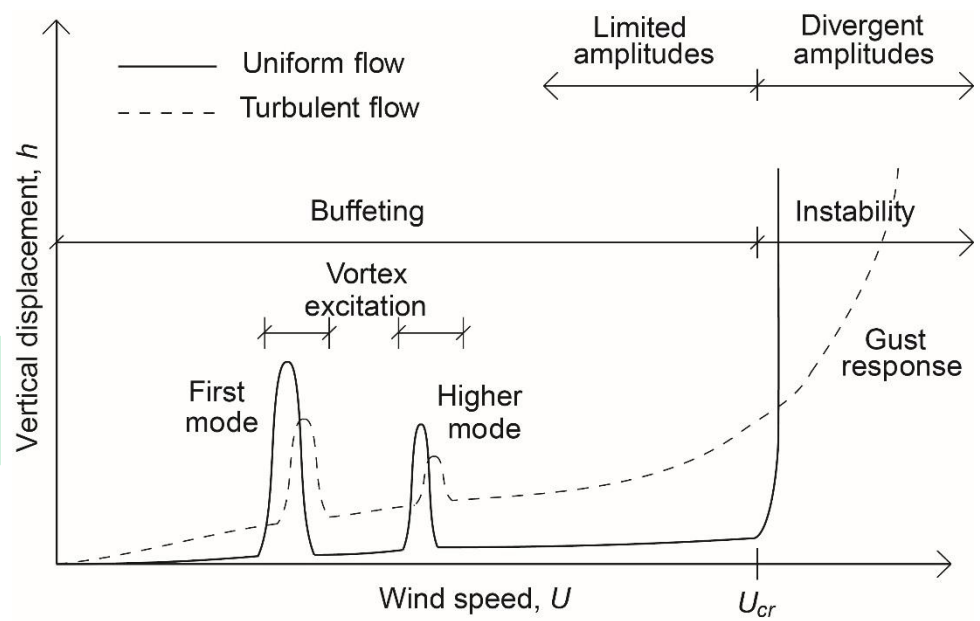
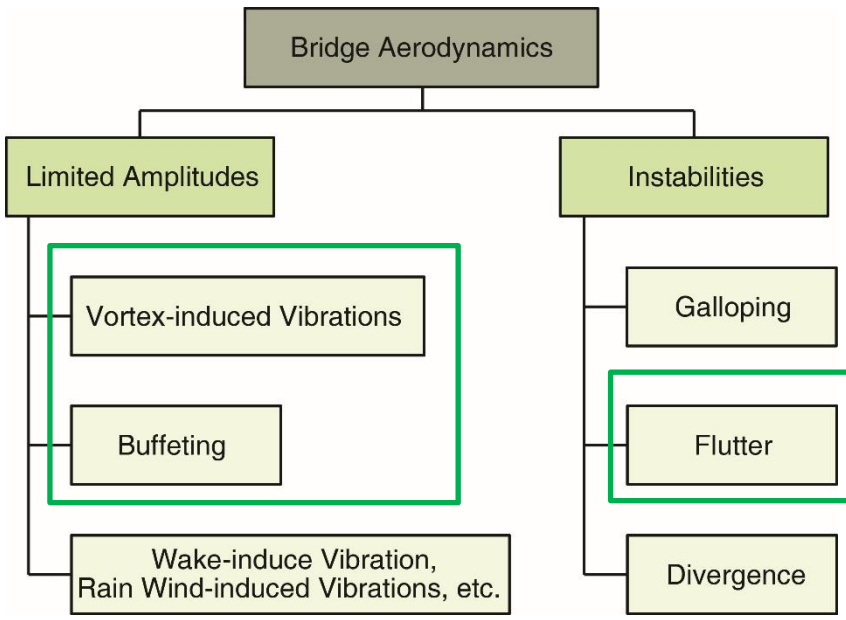
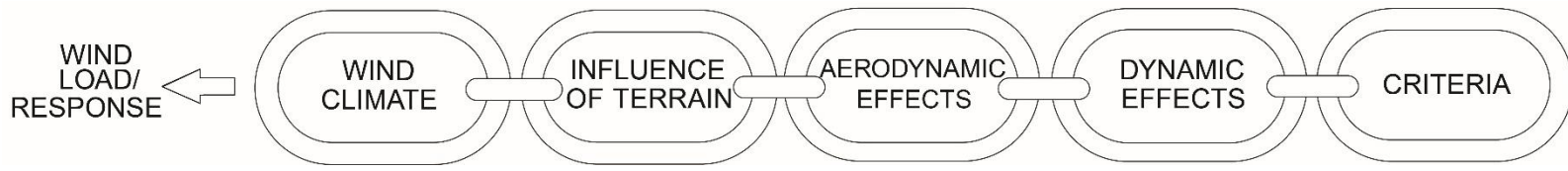


## Motivation

- Long and slender structures
- Light weight
- Low structural damping



# Wind Engineering (Design)



# Vortex Induced Vibrations

CONTESTABILE, Carlo

Università degli studi di Genova, Italy

KENÉZ, Ágnes

Budapest University of Technology and Economics, Hungary

PIERINO, Sonia

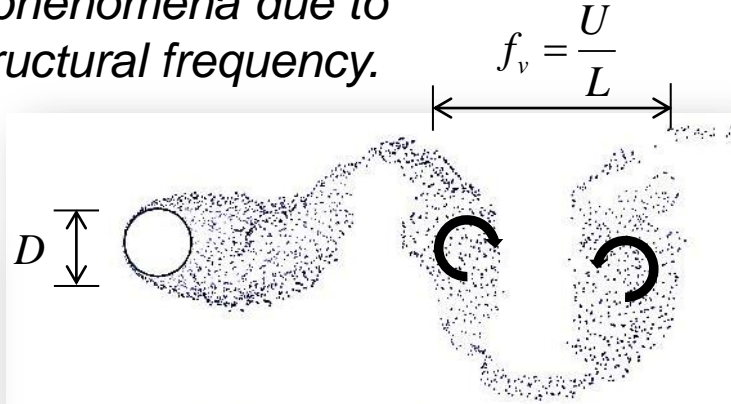
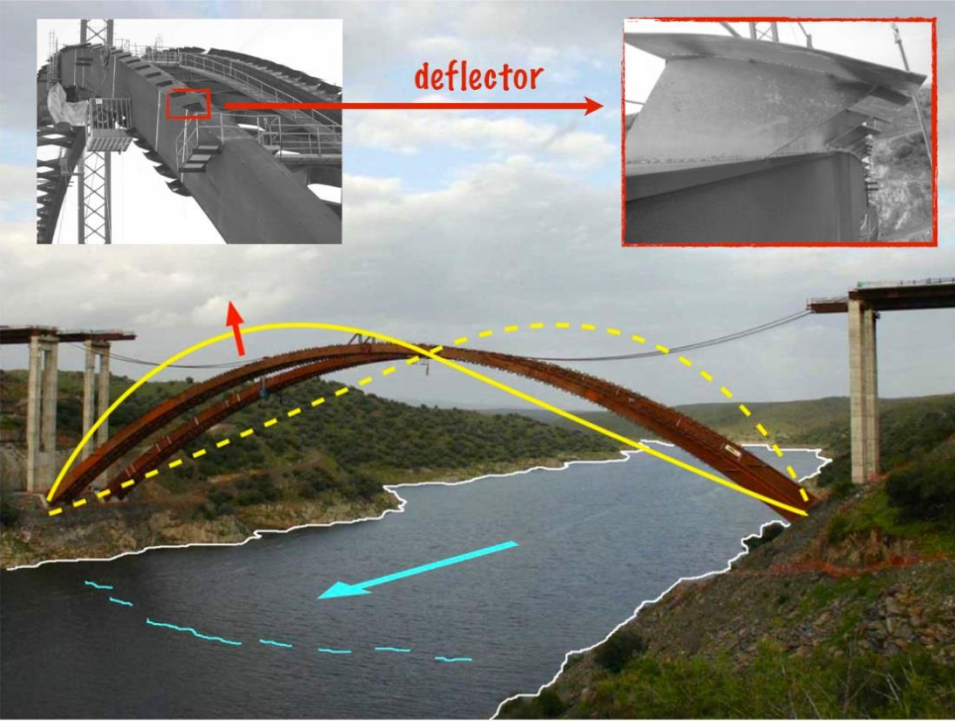
Università degli studi di Genova, Italy



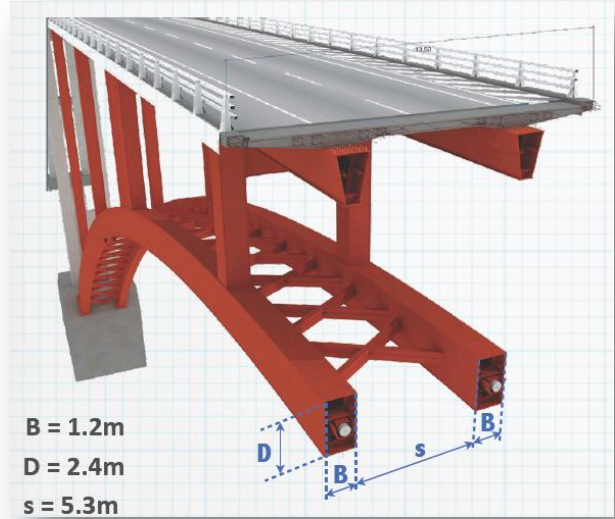
# Vortex Induced Vibrations

Vortex-induced vibrations (VIV) are resonance phenomena due to matching vortex shedding frequency and the structural frequency.

Lock-in → Resonance  $f_v = (0,8 - 1,2) f_n$



Self-limiting phenomenon [1]



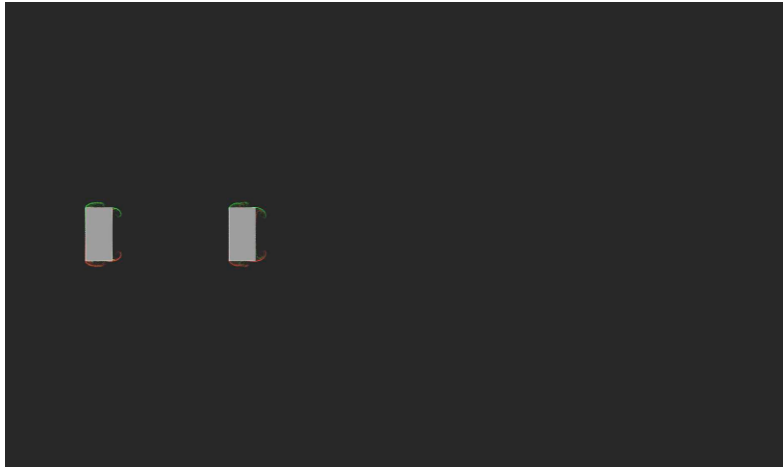
Motion of the Alconétar Bridge (upper) and its parameters (right) [1]

[1] Tajammal Abbas (2016), PROJECT 6: WIND-INDUCED VIBRATIONS OF LONG-SPAN BRIDGES, Bauhaus Summer School 2016

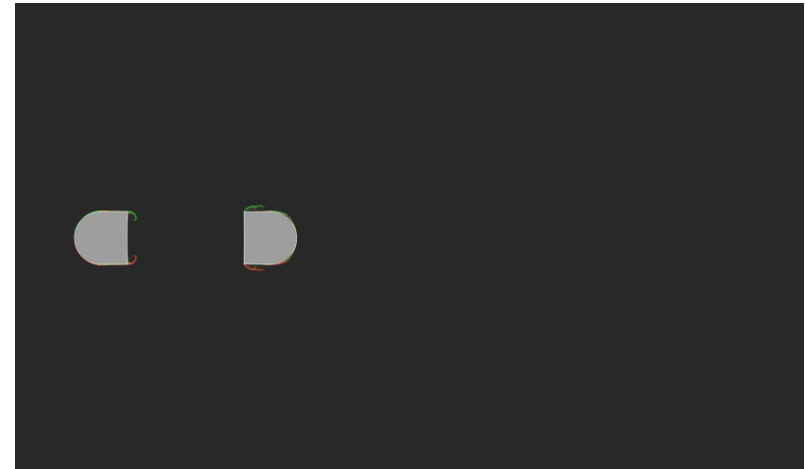


## Vortex Induced Vibrations

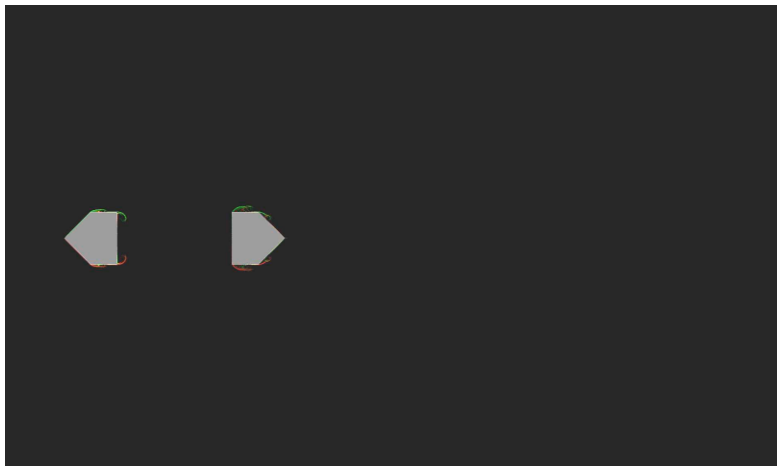
Rectangular cross-section



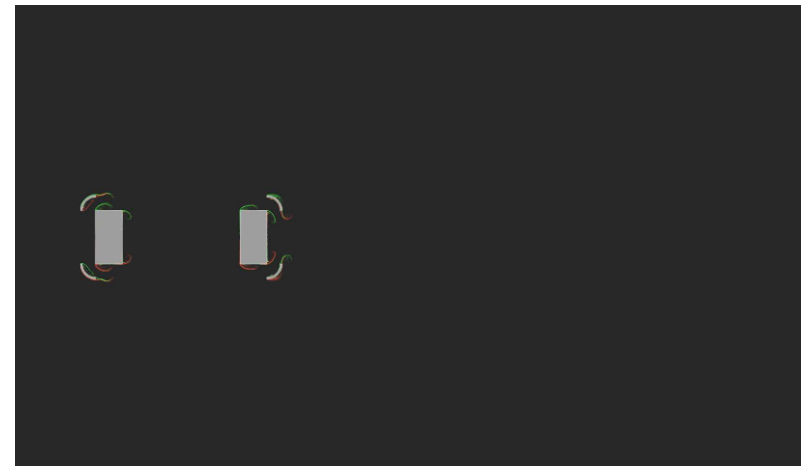
Rectangular + half-circle cross-section



Pentagon cross-section



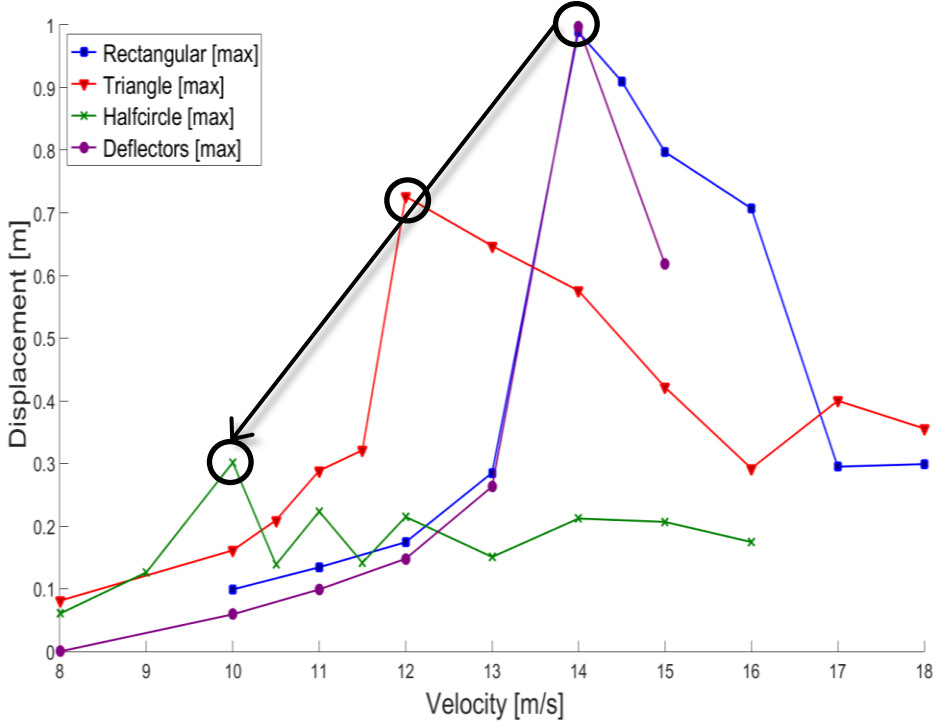
Rectangular + deflectors cross-section



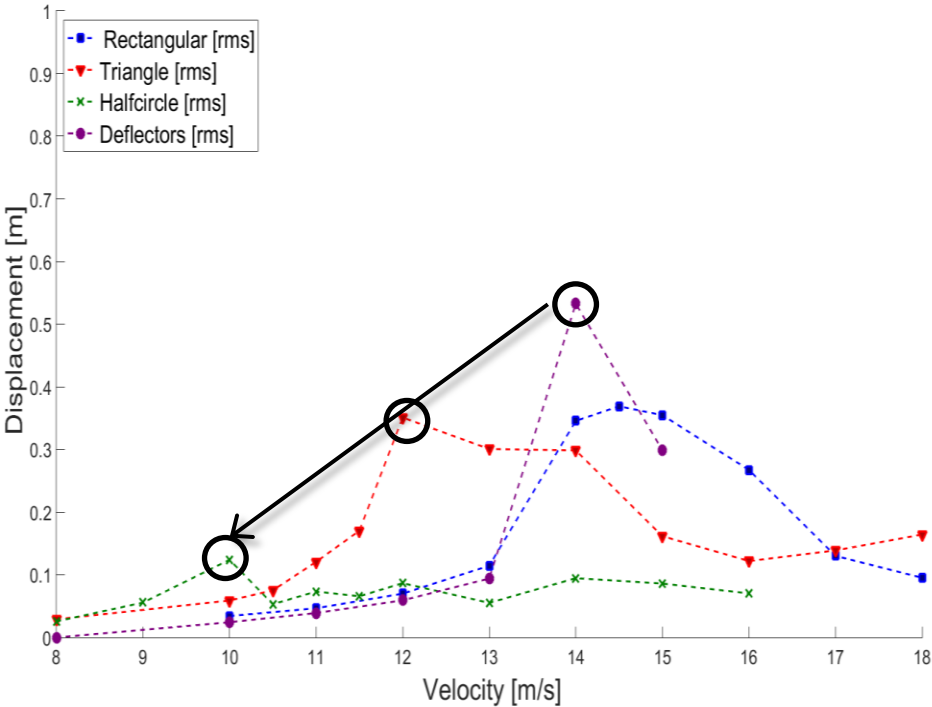


# Comparison of displacements

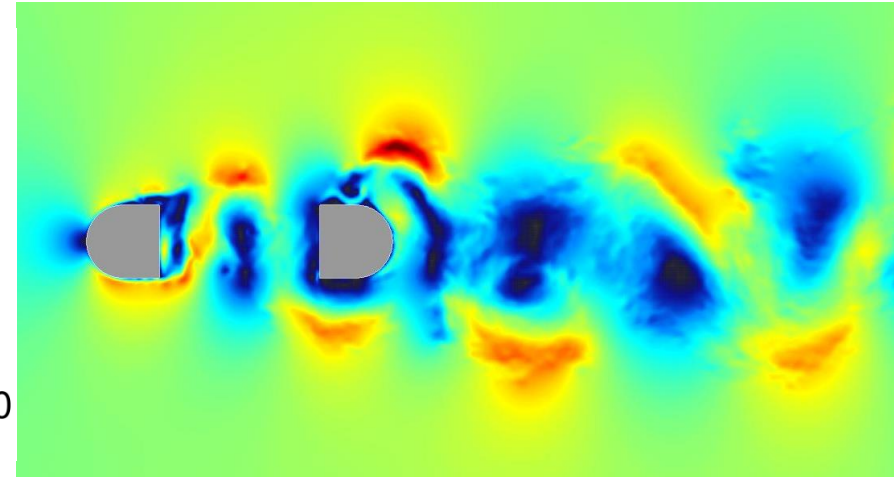
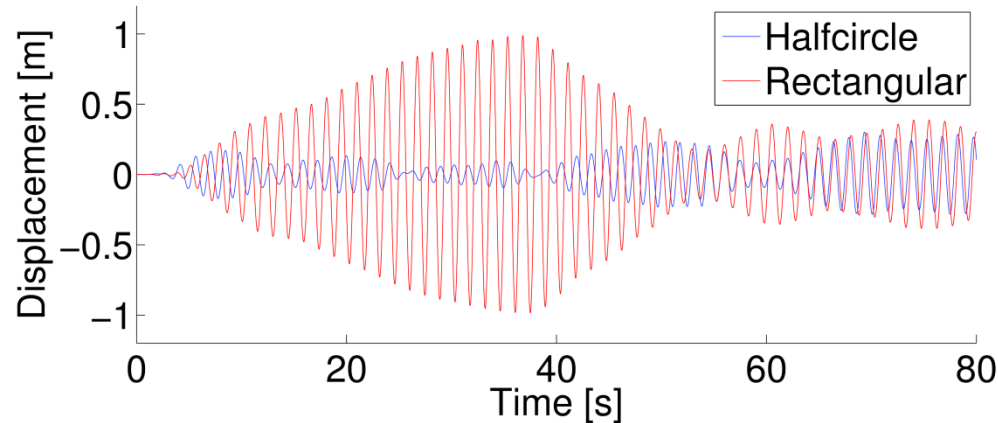
## Maximum displacement



## RMS displacement



## Optimized shape – Halfcircle



## Conclusion

- VIV is self-limiting phenomena, not destructive
- Fatigue problems, comfort issues
- Influence of geometrical shape on VIV

## Outlook

- TMD can be studied
- Moveable flaps





# Buffeting analysis

ABDOLLAHI, Hossein

Daneshpajoohan Institute of Higher Education, Iran

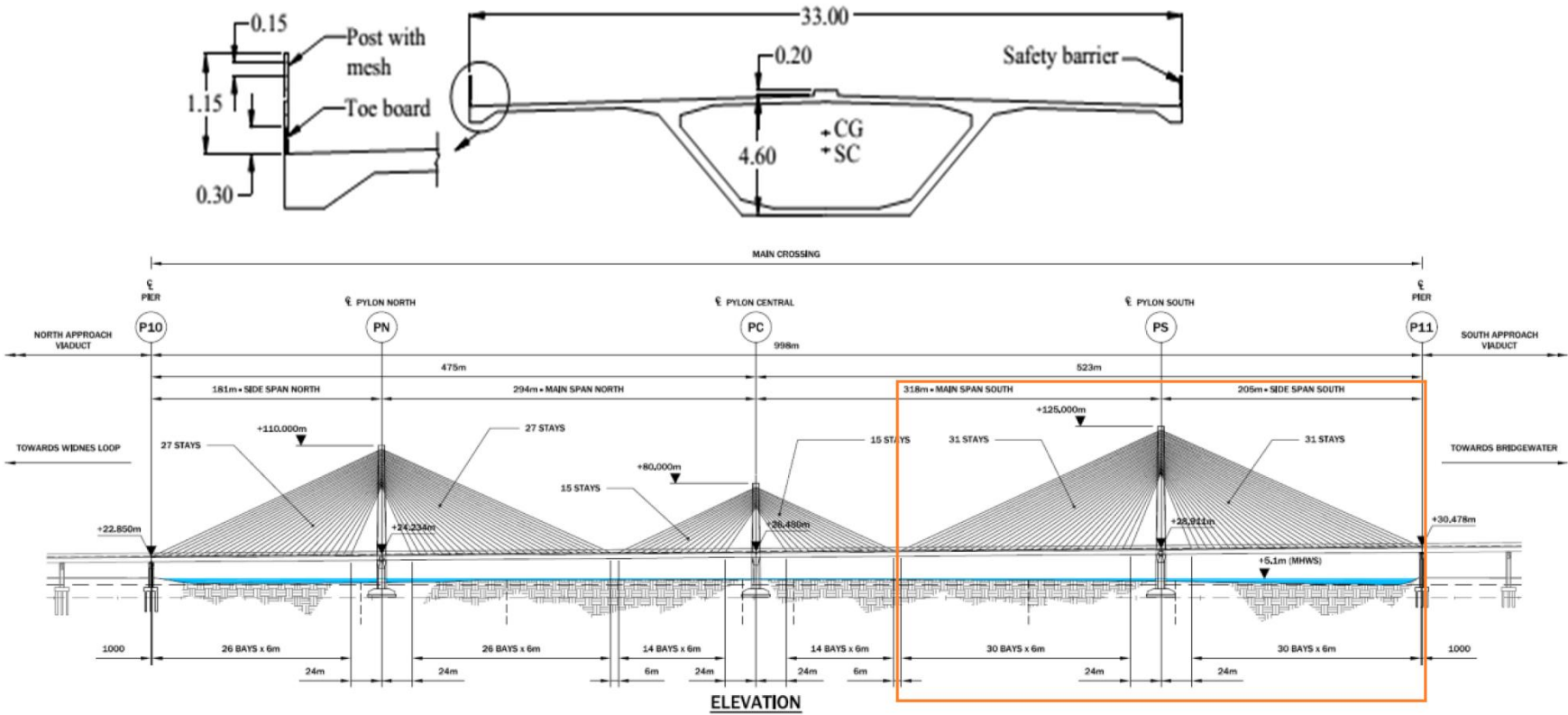
SHAHGHOLI, Farshad

Daneshpajoohan Institute of Higher Education, Iran

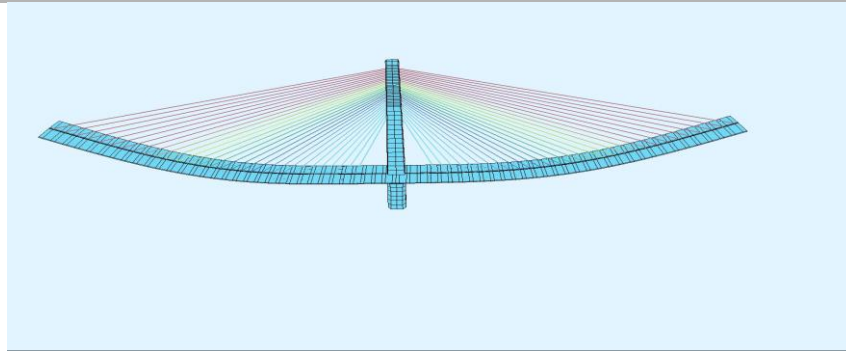
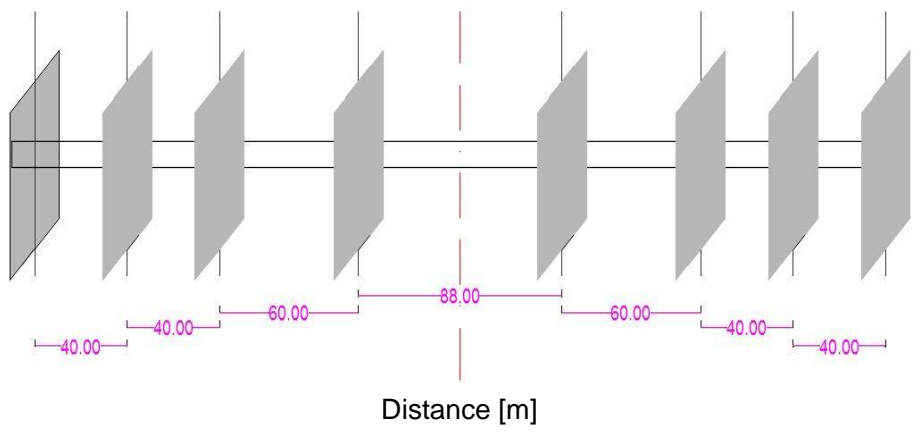
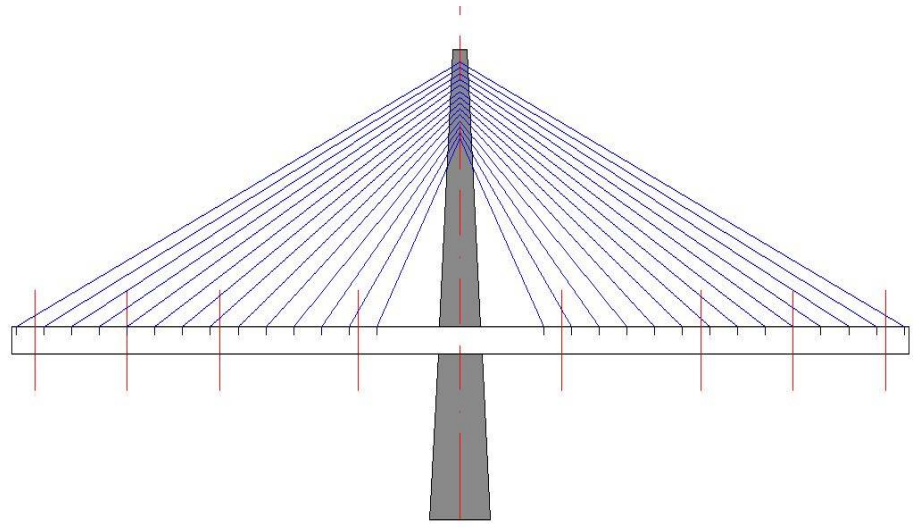


**Buffeting** excitation is caused by fluctuating forces induced by inflow turbulence in the wind field.

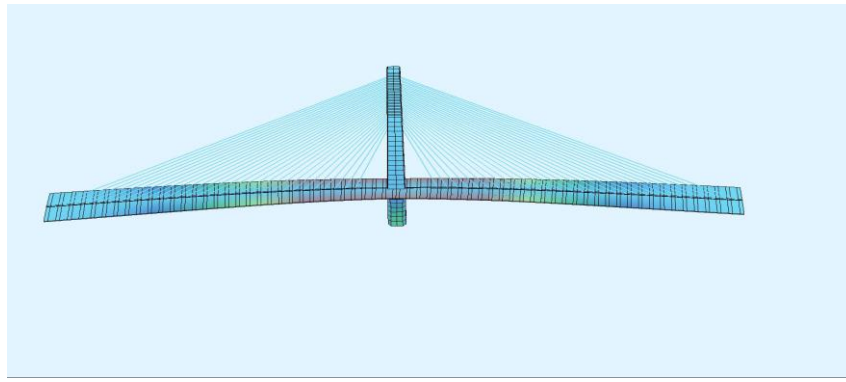
## Reference object



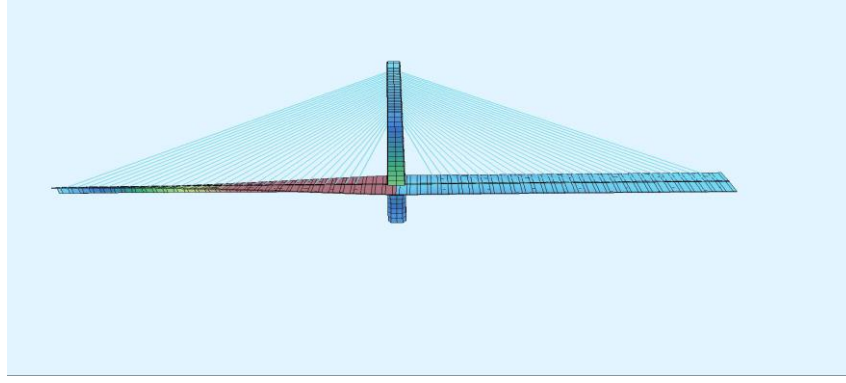
### Structural system and discretization



$f_v = 0.401 \text{ Hz}$

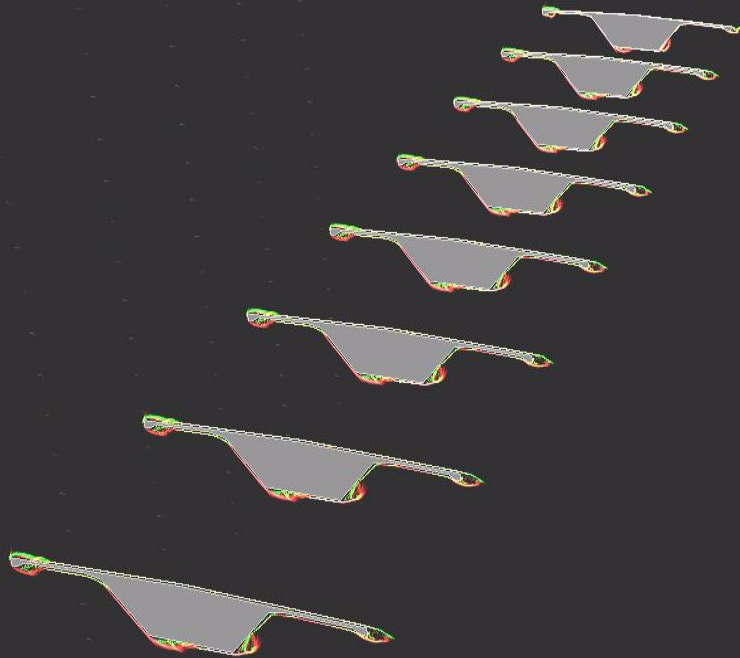


$f_L = 0.444 \text{ Hz}$



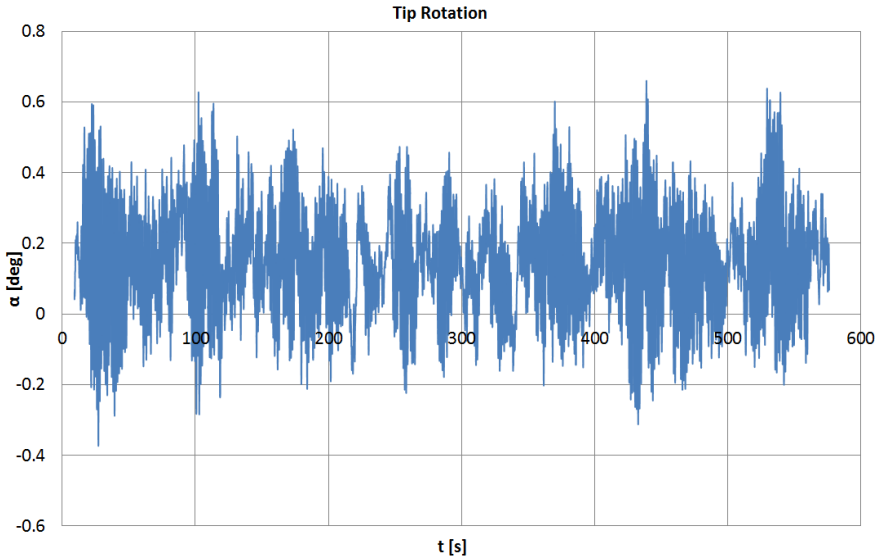
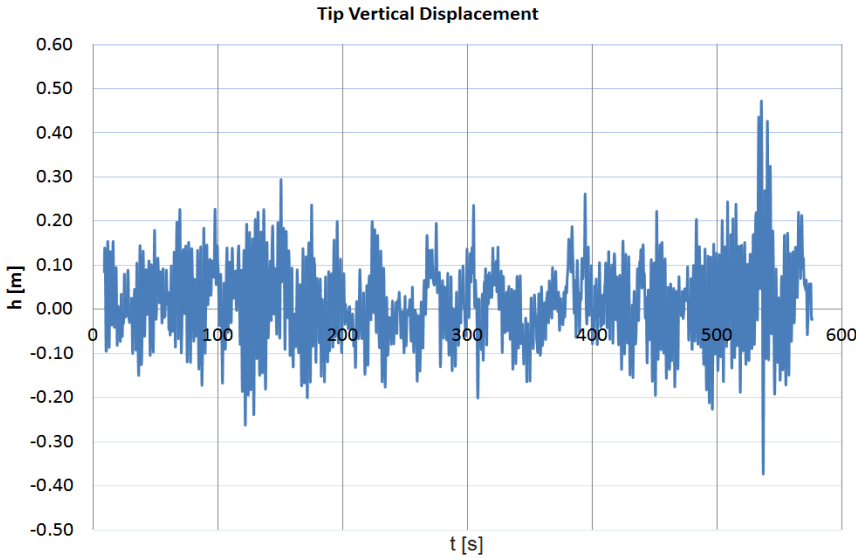
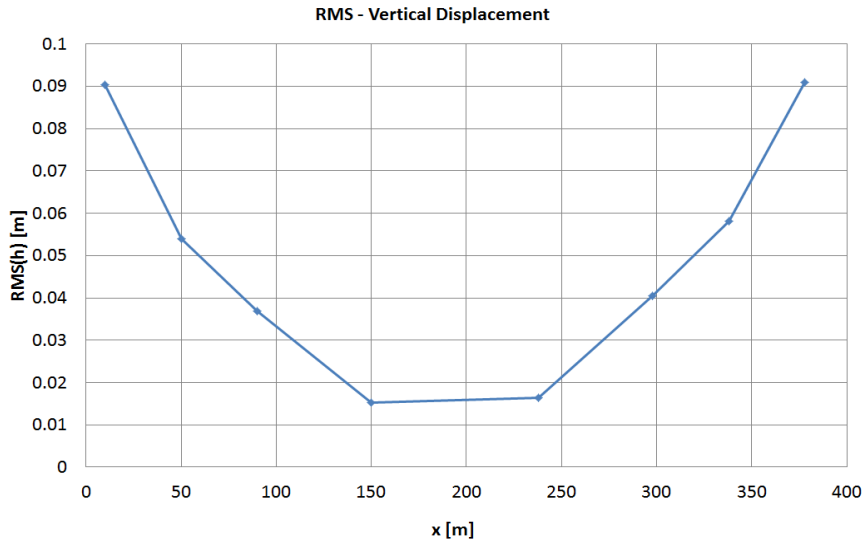
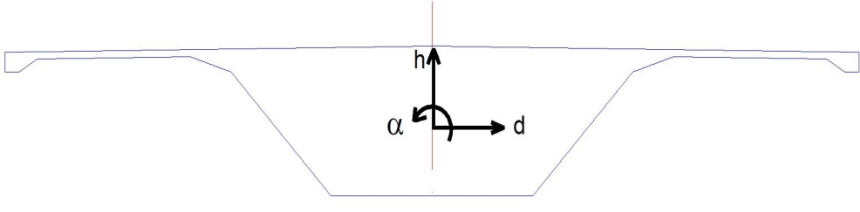
$f_T = 0.913 \text{ Hz}$





## Structural Response

$U=55 \text{ m/s}$ ,  $Tu=Tw=10\%$ .



# Flutter

IVANOVIĆ, Nikola  
University of Belgrade, Serbia

LUČIĆ, Sanda  
University of Josip Juraj Strossmayer Osijek, Croatia

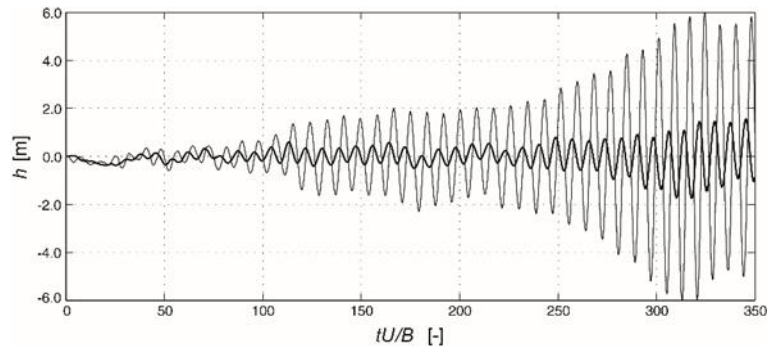
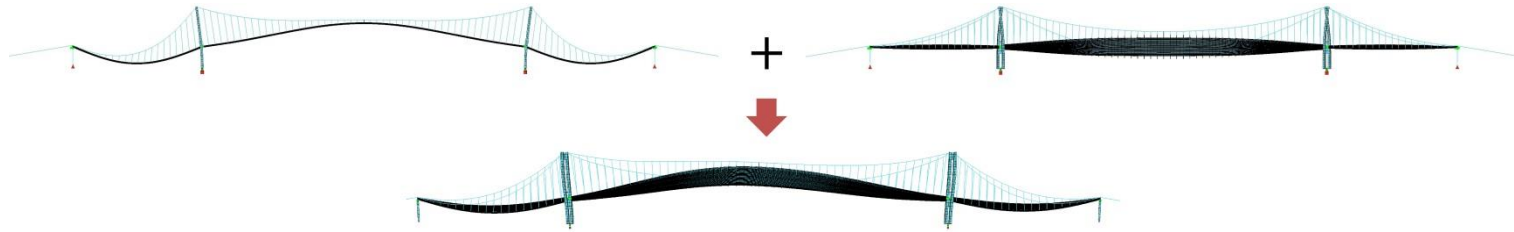
ŠPIRIĆ, Stefan  
University of Belgrade, Serbia





## Flutter

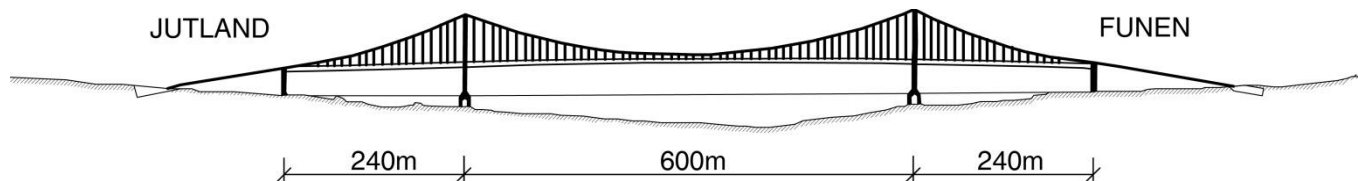
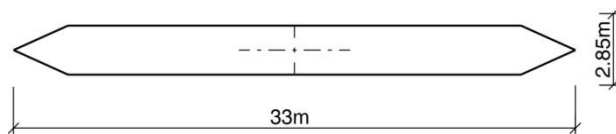
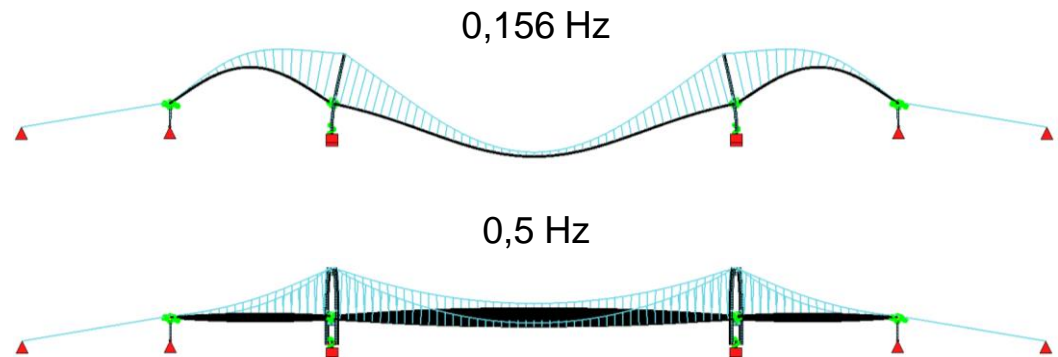
- Coupling of vertical and torsional oscillating mode
- Aeroelastic instability at higher wind speeds
- Can cause ULS if not taken into account



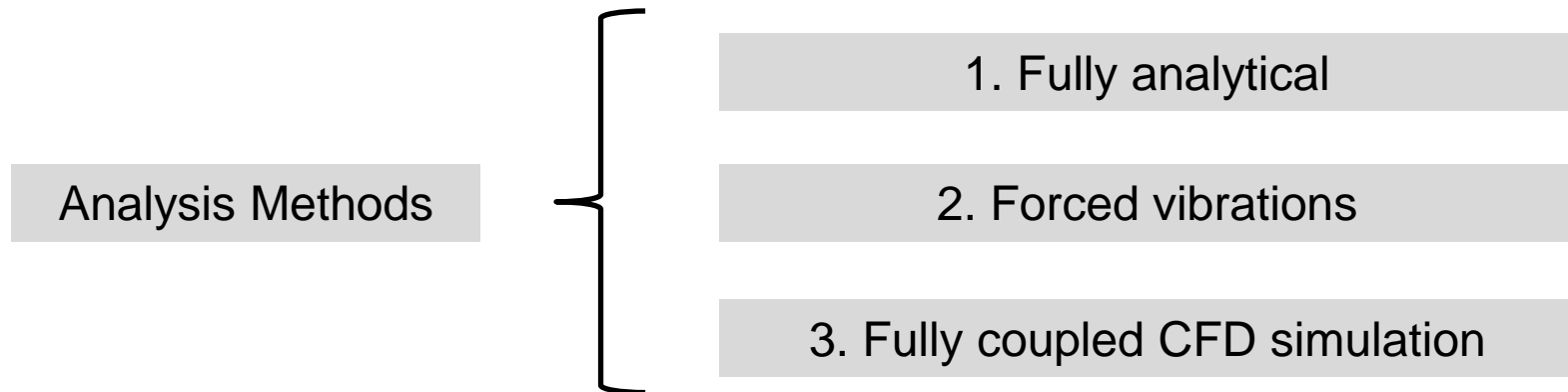
## Lillebælt Suspension Bridge, Denmark



- Total length 1080 m
- Steel box girder deck section



## Analysis Methods

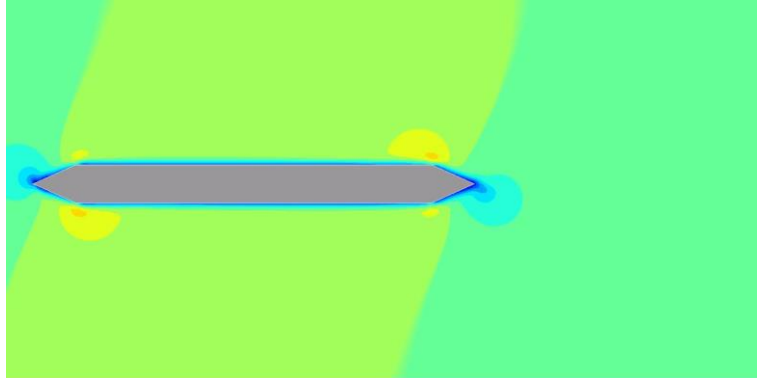


1. Potential flow theory, analytical aerodynamic and structural model
2. Numerical aerodynamic model and analytical structural model
3. Numerical aerodynamic and structural model

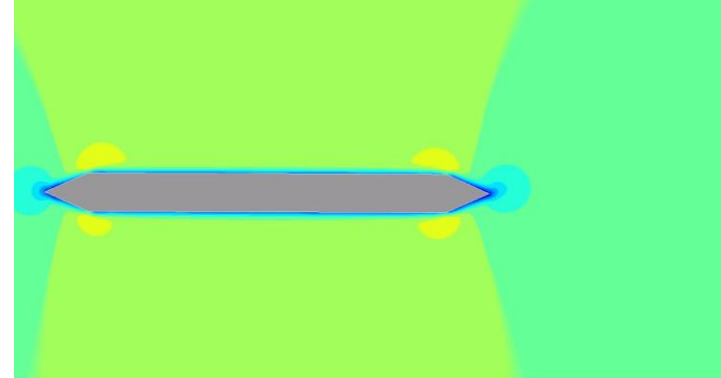


# Forced Vibrations

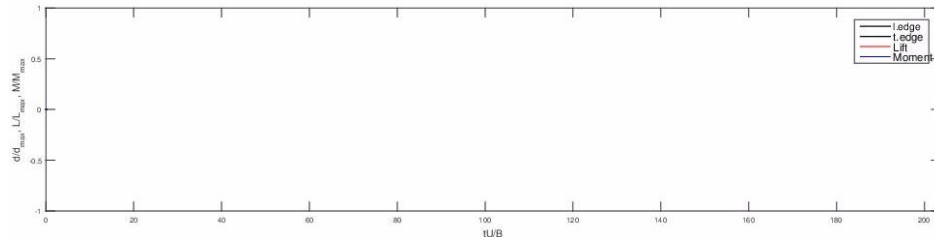
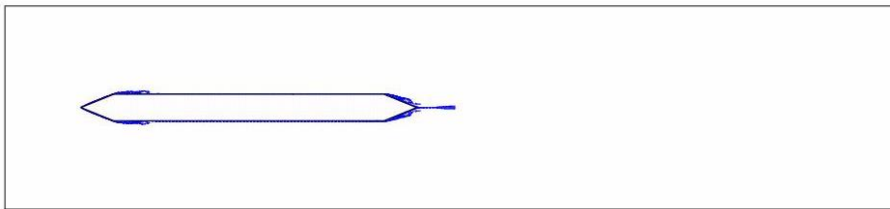
Heave



Pitch



Fully coupled CFD – single slice



Critical wind velocities

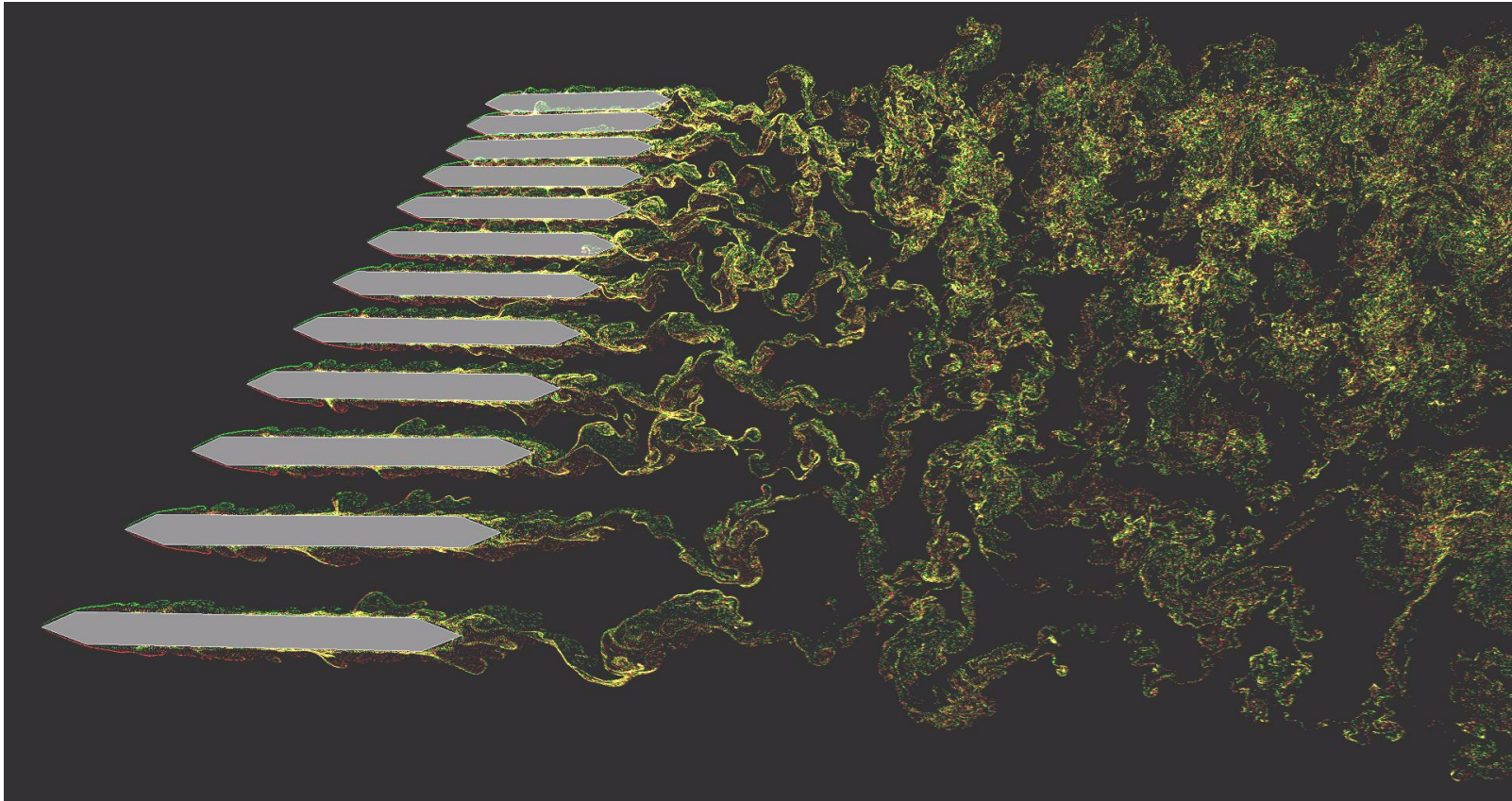
$$U_{cr,Theodorsen} = 94 \text{ m/s}$$

$$U_{cr,Scanlan} = 101 \text{ m/s}$$

$$U_{cr,CFD} = 98 \text{ m/s}$$



## Fully coupled CFD – Multi slice





# Conclusion

